The following document was prepared in 2008 by Coffey Geotechnics Pty Ltd for Coopers Gap Wind Farm Pty Ltd. It is important to note that this document was prepared for a client separate to AGL and for a wind farm layout and specification that is different to that which is currently proposed as a part of this Community Infrastructure Designation. This document has been included as a reference document to this IAR purely for the purposes of providing further detail behind some aspects of the study previously undertaken on the site, and is intended to be read only where referenced within the IAR Volume One document.



# PRELIMINARY GEOTECHNICAL ASSESSMENT

Coopers Gap Windfarm

GEOTKPAR01416AA-AB 7 April 2008

Investec Bank (Australia) Limited



7 April 2008

Investec Bank (AUSTRALIA) Limited 300 Gilles St Adelaide SA 5000

**Attention: Mr Jim Trenery** 

Dear Jim

**RE: Preliminary Geotechnical Assessment** 

**Coopers Gap Windfarm** 

Pleas find following our preliminary geotechnical assessment for Coopers Gap Windfarm. We trust that this report will satisfy your requirements for the time being. We would be pleased to work with you to develop a scope of work for a more detailed geotechnical assessment.

Should you require further information or assistance, please contact the undersigned.

For and on behalf of Coffey Geotechnics Pty Ltd

Ian Shipway

Principal

Distribution: 3 copies Investec Bank (Australia) Limited

1 Copy Coffey Brisbane Library

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#### important information about your coffey report

#### **Drawings**

Drawing 1: Site Plan

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Appendix A: Field Investigation Data

#### 1 INTRODUCTION

Coffey Geotechnics has conducted a preliminary geotechnical assessment of the proposed site for the Coopers Gap Windfarm in south east Queensland. The assessment was conducted on behalf of Investec Bank (Australia) Limited with the following intentions:

- Identify and generally characterise the different geological units within the extents of the project;
- Make a generalised preliminary assessment as to the likely founding depths for Wind Turbine Generator (WTG) footings in the different geological units; and,
- Make a preliminary assessment of geotechnical constraints that could affect the construction of access roads and other infrastructure.

This report presents the results of our assessment, addresses the items above, and provides information on some other geotechnical aspects discussed on site with Mr Jim Trenery of Investec Bank. Also included are design parameters for guy anchor blocks which we understand are necessary for the construction of wind monitoring towers.

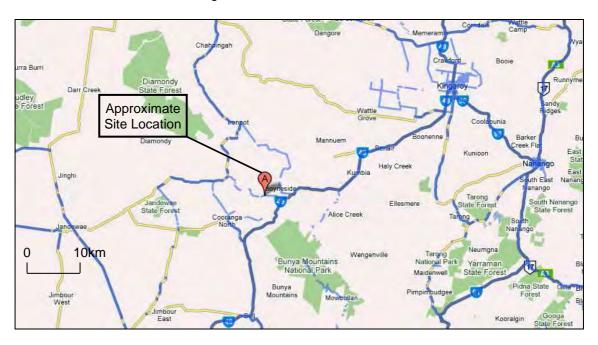


Figure 1.1. General Site Location

#### 2 PROJECT DESCRIPTION

The site for the proposed windfarm is located off Niagara Road in Boyneside which is about 70 km south west of Kingaroy as indicated on Figure 1.1. Topographically, this vicinity is characterised by a series of north west to westerly trending ridgelines along which up to 147 wind turbine generators are to be distributed. Drawing 1 shows the general layout of wind turbines at the time of the assessment.

The turbines will be founded through steel reinforced concrete footings which generally found at up to 3m below surface level. Footings may be either of the gravity variety, that is, possessing sufficient weight to resist the overturning moments developed by the mass and operation of the turbine, or may be smaller footings that provide resistance to the forces through ground anchors.

Roads connecting the wind turbine sites to the surrounding public road system will be required to allow construction, and also to provide access for maintenance vehicles for the design life of the wind turbines. Construction traffic will include large, heavily laden trucks. Experience indicates that although such vehicles are generally limited to grades of less than 10%, they can often traverse short distances with grades up to 20% in good weather conditions with assistance.

In addition to the access roads, the WTGs will need to be linked by a series of electrical cables which are generally layed in trenches with a depth of at least 1m below ground surface level.

#### 3 WORK CONDUCTED FOR THIS ASSESSMENT

The work conducted for this assessment comprised the following:

- o A desktop review of available geological information.
- A site visit by a Principal Engineering Geologist on 21 and 22 May 2008. During this site visit, twenty two test pits (designated CG1 to CG22) were excavated using a rubber tyred backhoe to allow assessment of typical subsurface features and limited geotechnical assessment.

The test pits were distributed over several of the ridge lines upon which the windfarm will be developed. These ridge lines have been designated A to I on Drawing 1 to assist with referencing in this report. The intent behind excavation of the test pits was to provide general characterisation of the range of ground conditions within the different ridge lines, rather than to investigate the specific sites of wind turbines.

The test pits were excavated to refusal or in several cases were terminated at depths below 2m depth where it was considered that no further useful information would be gained by proceeding further. The backhoe that excavated the test pits was a modern Case machine fitted with a 600mm wide bucket with tiger teeth in near new condition.

Some parts of the site, particularly those areas to the north west, could not be assessed due to access restrictions as discussions between the developer and land holders were still ongoing at the time of the site visit.

In addition to the main site area, a visit was made to a location which could be a potential source for construction materials, and two cuttings on Niagara Road where subsurface conditions were exposed over a depth of about 6m. Drawing 1 shows these locations.

#### 4 GEOLOGICAL FRAMEWORK & SURFACE OBSERVATIONS

#### 4.1 Geology

Published geological information shows that the vicinity of the proposed windfarm is mainly underlain by basalt bedrock of Tertiary age. Basalt is a dark, fine grained, volcanic rock that was deposited as distinct layers of lava which could vary in thickness from a few metres to nearly a hundred metres. Although basalt is usually very strong in its fresh state, it weathers relatively quickly (in geological time scales) to form dark brown and red clay soils. The pattern of basalt weathering often leaves "corestones" or boulders of strong rock within a soil matrix. This rapid weathering, coupled with the mode of deposition in successive layers can often lead to a situation where one layer weathers to a soil and then is overlain by a new lava flow, which is itself weathered and partially eroded. The result of this is that terrain formed in basalt can often have relatively weak material underneath strong rock, a situation that is relatively uncommon outside of basalt terrains.

There are areas of the site to the north east that are underlain by older sedimentary rocks. These materials are not discussed in this report as those areas could not be visited during field work due to the access constraints outlined previously.

#### 4.2 Surface Conditions

The development area comprises a series of west to north west trending ridge lines which rise to as high as 850m above sea level. The area has generally been cleared, with only scattered trees remaining. Grazing is the predominant land use along the ridge lines and slopes. Ground cover is mainly long grass with some prickly pear and small shrubs. Clay soils are generally evident at the surface, with zones of scattered rock fragments and rare areas of rock outcrop. Where outcrop is visible at the surface it usually comprises fractured basalt which would break into large fragments on excavation.

Slopes vary greatly in steepness from very shallow to angles steeper than 20°. The density of contour lines on Drawing 1 provide a reasonable comparison of the steepness of different areas. The current access to Ridges F and G required us to traverse slopes of up to 10° (about 17%). Access to Ridge H uphill of test pit CG21 would require a long section of slope of 15° (27%) to be traversed. Current access to most of the other ridges assessed involves traversing slopes of not more than 7° (12%).

#### 5 SUB-SURFACE CONDITIONS

#### 5.1 Observations in test pits

The test pits revealed a profile comprising variably weathered basalt which formed a profile as outlined in Table 1. More detailed descriptions of materials are provided on the Engineering Logs included in Appendix A, which also includes sheets explaining the meaning of the terms used in the descriptions.

TABLE 1. SUMMARY OF MATERIALS OBSERVED IN TEST PITS

Depth Range	Generalised Material Description
Not present at many locations. Generally around 0.5m thickness but over 3.3m in CG21 on Ridge H	Unit 1 Very stiff and hard, brown and red brown clay. Clay is generally of high plasticity with some medium plasticity materials.
Not present at some locations. Generally about 0.5m thick, but varies.	Unit 2 Very stiff and hard gravelly clay or very dense clayey gravel usually with some large rock fragments over 100mm in size. Generally medium or high plasticity clay.
Present only at a few locations.	Unit 3  Basalt fragments in a soil matrix. Rock fragments varying in size from 100mm to 500mm in size in a matrix of stiff to hard clay. In some locations fragments are much smaller than others.
Found in nearly all test pits.	Unit 4 Weathered basalt bedrock. Varies in degree of weathering. Generally high to very high strength but with numerous soils strength zones between sections of rock. In most cases the backhoe bucket refused after penetrating about 0.5m, but in some cases penetrated over 1m into predominantly rock strength material. The spoil size of the excavated material varied significantly from about 100mm to as large as 600mm.



Figure 5.1 shows the profile from test pit CG8 on Ridge C.

Figure 5.2 Profile in test pit CG8 showing clay over basalt.

The difference between Unit 3 and Unit 4 relates to the assessed quantity of rock strength material, with Unit 4 comprising mainly rock with some soil zones. Figure 5.1 shows a comparison of spoil between two of the test pits to give some idea of the range of sizes of excavated spoil observed.

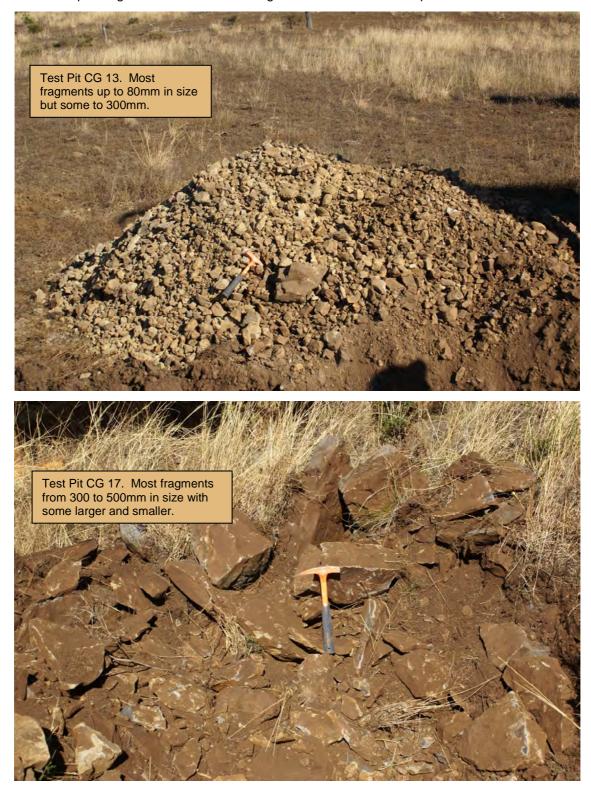


Figure 5.1. Comparison of Spoil from Two Test Pits

#### 5.2 Observations in Cuttings

The two cuttings revealed highly complex, variable conditions over short distances. Some parts of the cutting face exhibited deep soil strength zones, whilst other areas were rock from near the surface. The materials were generally weathered such that the block size in disturbance was generally up to 100mm, but with some blocks to about 600mm in size. These conditions are generally consistent with those revealed in the test pits. Figure 5.3 shows a typical cutting face. Note the spoil forming at the base which is a mixture of soil and rock strength material.



Figure 5.3. View of South Western Side of Road Cutting

#### 5.3 Observations at Quarry Site

Conditions at the potential quarry site are quite different to those on the surrounding hillsides. In this location the near surface materials comprise variably sized fragments of fresh, very high strength basalt as shown on Figure 5.4. These conditions have formed through the collapse of large, joint defined "columns" of basalt which make up the adjacent hillside. Lava flows from which the basalt was formed often develop such continuous near vertical joints as they cool. At this location the adjacent creek appears to have undercut the base of the slope, exposing the columns and allowing collapse.

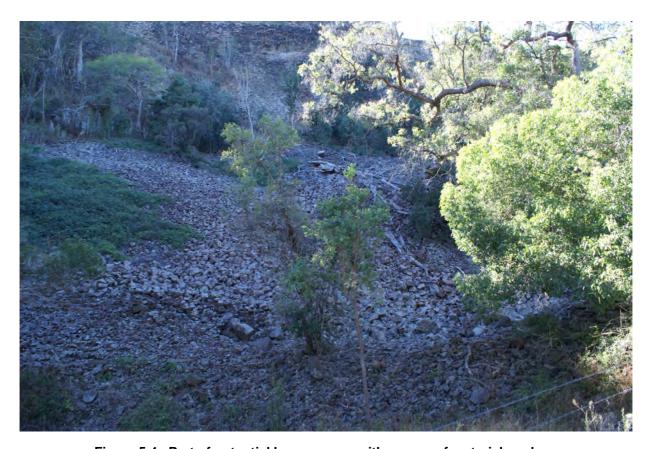


Figure 5.4. Part of potential borrow area, with source of material upslope

#### 6 PRELIMINARY GEOTECHNICAL ASSESSMENT

#### 6.1 Founding of Wind Turbine Generators

The WTGs will be founded on either "gravity" type near surface footings or reinforced concrete footings restrained by ground anchors.

For the anchored footing design to be appropriate for a specific WTG site the near surface materials must have appropriate strength and stiffness and there must be suitable rock at depth over the bond length of the anchors which would be at depths greater than 10m below the base of the footing.

Basalt provides specific challenges for an anchored footing system because of the potential for less strong materials underlying strong near surface materials. At this stage, the near surface investigations conducted so far have not revealed such conditions but they may still present undetected.

The main parameters pertinent to design of footings of both kinds are elastic modulus, serviceability bearing pressure and ultimate bearing capacity of the near surface materials. A key parameter governing design of an anchored footing is the allowable bond stress of the ground to grout bond at depth.

Based on the available information and our experience of the design requirements at other sites, we consider that at least some of the WTG sites would be able to satisfy the near surface criteria for an anchored footing. Deeper drilling to provide a greater understanding of the geology of the successive

lava flows comprising the ridge lines would be needed to make an assessment of the anchor bond zones. The quantity (and therefore the cost) of such investigation may be greater than usual for non basalt sites.

Based on the information available, we are of the opinion that suitable founding strata would be present at most locations between 0.7m and 1.5m depth and that an economic gravity footing could be designed for these ground conditions with the possible exception of the deep clay soils intersected by test pit CG21 on Ridge H. Note that these conditions have been found in only a singe test pit, and that this pit was excavated on the flanks of the slope rather than the ridge line because of access constraints.

#### 6.2 Excavation Conditions

#### 6.2.1 General Excavations for Footings and Roads

The test pits have demonstrated that the majority of materials on site can be excavated down to about 1m depth or a bit less using a backhoe. Note that this was achieved using good equipment (detailed previously) in a confined trench. Large hydraulic excavators (ie, 30 tonne or greater) should be able to penetrate further, particularly in a larger bulk excavation. However, the variably weathered nature of the bedrock and the presence of rock outcrop in some locations suggests that there will be some zones where excavation using excavators would not be practical or economic. Large bulldozers or even blasting could be required if such zones cannot be avoided.

#### 6.2.2 Excavation of Cable Trenches

During past assessments with respect to cable trenches and the like, we have developed a table of excavation classes as shown on Table 2.

**TABLE 2: EXCAVATION CLASS DEFINITIONS** 

Class	Typical Material Descriptions	Likely Type of Machine to be Used
I	Soil; soil & "very weak rock" (i.e. typical Unconfined Compressive Strength (UCS) < 2 MPa)	Cleveland Bucket Wheel Excavator
II	"Weak rock" (i.e. typical UCS<40MPa), open closely jointed "strong" rock.	Vermeer Chain Trencher
III	Tight jointed "strong rock"; variable rocky ground.	Tracked Excavators & Hammers
IV	Massive "strong" rock.	Blasting or Heavy Use of Rock Hammers

The conditions revealed by the test pits (and inferred by surface observation) generally fall into either Class II or Class III with the majority being "variable rocky ground" (Class III). Whilst a Vermeer trencher may be successful in some parts of the development, a particular challenge is posed by the large, high strength boulders within the soils which allow the trencher to get up to speed before bringing it to a rapid halt, often resulting in broken teeth and slow repairs. Taking this into account, we consider

that large excavators would be a more economical alternative for excavating cable trenches. Note that even using such equipment it may be necessary to divert trenches around some zones of stronger material or alternatively resort to blasting or heavy rock hammers for short sections.

#### 6.3 Road Construction Issues

#### 6.3.1 Geometrics and General Issues

The philosophy behind road construction should be carefully considered prior to embarking on the project. Our understanding is that the roads are required to be serviceable during construction under relatively heavy traffic and for the in-service period under infrequent light traffic. An all weather, easily maintained, unsealed road is usually adopted. Our experience suggests that as the majority of traffic will be experienced during the construction period there is little point in trying to construct the final pavement surface at the outset. It will often be more practical for the contractor to form the road and maintain/resurface during the construction period as required by very short term traffic requirements. The finished surface would be trimmed etc on completion of construction of the turbines and other infrastructure.

In previous projects we have been advised that the maximum grade trafficable by the required construction traffic is 10%, however we are also aware that in practice short sections of road at up to 20% grade have been used. Larger trucks may require assistance to get through these steeper sections and they may not be passable during wet weather.

As noted previously, the current access to several of the ridge lines (F, G and H) involves traversing slopes significantly steeper than 10% and at some places steeper than 20% grade. It is likely that there are other unobserved locations that will be similarly steep. However it is possible that there are other practical routes to access many of these sites where grades would not be as steep. These will need to be carefully assessed, and it may be worth considering accessibility in detail when finalising turbine locations.

If large tracked cranes are required for construction, as is the case in many wind farms it may be necessary to construct access roads over 10m wide with tight tolerances on the maximum side slopes. Under these circumstances the steep terrain of the development area may provide an even greater challenge. Routes for roads will need to be carefully selected to limit the amount of cut required to meet the specification for cross fall.

#### 6.3.2 Materials for Road Construction

Construction materials for a serviceable road as outlined above would generally comprise a mixture of gravel, sand and clay/silt binders. Usually the maximum size of these materials would be about 80mm except for the case of certain weak rocks that break down under a grid roller. For unsealed roads such materials generally require slightly more plastic fines than a typical road base will usually possess. The less processing a material requires, the cheaper it will be. Therefore preference would be given to materials that can be excavated and used immediately, rather than those that require crushing and complex screening. An intermediate step would be basic screening to remove oversize particles.

In our opinion it may be practical to use some of the materials won during excavation of footings and small cuts for roads in construction of road pavements. The challenges posed by basaltic materials in this regard are the high plasticity of the clay component and the large blocks of strong rock that are likely to be won from some locations as shown in Figure 5.1. This material would certainly require

screening to remove larger blocks. These could be crushed by mobile on site crushing plant if greater volumes of material are required than can be provided by screening alone. In some areas (ie, Ridges A and C) there is a high proportion of clay which may make the materials unusable unless the materials can be selectively won to remove the more clayey components.

#### 6.3.3 The Potential Quarry Site

Economics will dictate that carting material for road construction from off site would only be contemplated if insufficient suitable material can be won on site during construction. Materials observed near the surface of the proposed quarry site comprised fragments of very high strength basalt (varying from gravel to over 0.5m in size) that appeared to be suitable for crushing to make good quality road material. Whilst the material appears generally suitable there are a number of issues which may affect the practicality of this source. Firstly, the mode of collapse of the basalt columns has formed very steep slopes adjacent to the hillside which may be readily subject to instability during extraction of resource. The extraction plan would need to be carefully considered to maintain the safety of those working on site.

Another aspect is the access road to the quarry site, a small section of which is shown on Figure 6.1. As shown this is an unsealed road in poor condition with many twists and low overhead clearance in several locations. It is likely that the road would need to be upgraded if significant truck traffic was considered. It is possible that the road may not be trafficable by larger semi-tippers even after upgrading.



Figure 6.1. Small section of proposed quarry access road

#### 6.4 Founding of Guy Anchor Block

Test pits CG8 and CG9 were excavated at the location of a proposed wind monitoring tower to provide information on the founding of the guy anchor blocks. The profile revealed by the test pits is that shown in Figure 5.1 and comprises clay and gravelly clay to about 0.6m (Unit1/2) overlying moderately weathered basalt (Unit 4). Based on our observations in the test pits and experience with similar materials we consider that the design parameters given in Table 3 may be adopted.

TABLE 3. DESIGN PARAMETERS FOR GUY ANCHOR BLOCKS

Parameter	Unit 1/2 (Down to ~0.6m)	Unit 4
Bulk Unit Weight	19 kN/m <sup>3</sup>	22 kN/m <sup>3</sup>
Allowable Bearing pressure at 0.8 m depth	NA	1000kPa
Effective cohesion	5 kPa	20 kPa
Effective angle of friction	32°	40°

We understand that these parameters will be used to calculate the resistance provided by anchor blocks founded at about 1m depth. Note that during excavation of the test pits the pit walls were loosened during excavation as larger rocks were "plucked" from the sides leaving an irregular shaped excavation. The parameters above assume that the anchor blocks can be constructed with a good contact between the undisturbed ground on the side of the blocks and the concrete. If such contact cannot be effected in practice some parameters may need to be down graded. Further advice should be sort from a geotechnical consultant under these circumstances.

For and on behalf of Coffey Geotechnics Pty Ltd

Ian Shipway

Drawings

Appendix A

Field Investigation Data



CG1

Excavation No.

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

INVESTEC BANK (AUSTRALIA) LIMITED 21.5.2008 Date started:

Principal: 21.5.2008 Date completed:

**COOPERS GAP WINDFARM** IS Project: Logged by:

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE - 450mm Bucket Pit Orientation: Easting: 346041 m						041 m		R.I	L. Surface:							
excavation	dimen	sion	is: 3	3.5m ld	ong (	0.7m w	ide		Northing:	704	4000 m		da	tum:	AHD	
excavati	ion in	forn	nation			mat	erial sı	ıbstance								
method 1 7 8 9 9 9 1 8 1 9 1 9 1 1 1 1 1 1 1 1 1 1	support	- 1	notes samples, tests, etc	RL r	depth metres	graphic log	classification symbol	material soil type: plasticity or particle colour, secondary and mino	r components.		moisture condition	consistency/ density index	100 pocket 200 d penetro- 300 w meter		ructure and nal observations	unit geology
НВ	Z Z	Notice Observed			0. <u>5</u>		CH/CL	CLAY: High plasticity, dark brown of fine to medium basalt gravel.  CLAY & GRAVELLY CLAY: High brown) & low plasticity, gravelly clay fine to coarse basalt gravel; with a angular basalt fragments to 150m  HW BASALT: Dark brown and recto high strength; numerous XW sehard, red brown, high plasticity clay fragments up to 150mm (but main excavation.	plasticity clay (reay (pale brown) value trace to some m in size.	ed with	D-M	н			_	HW Basalt RS-XW Basalt Residual Soil
					1. <u>5</u>			Near refusal at 1.3m Test pit CG1 terminated at 1.3m							-	

ı									
- 1	method support				samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D disturbed sample		syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Excavation No.

Date completed:

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

22.5.2008

**CG10** 

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment	equipment type and model: CASE - 4						ket	Pit Orientation: Easting:	Easting: 346041 m			R.L	R.L. Surface:		
excavation	dime	nsio	ns: 3	3.5m l	ong (	).7m w	ide	Northing	: 70	44000 m	1	datı	ım: AHD		
excavati	on ir	nfor	mation			mat	material substance								
method  1  2 penetration		water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristicolour, secondary and minor component	S	moisture condition	consistency/ density index	100 pocket 200 penetro- 300 penetro- 400 meter	structure and additional observation	ട്ട unit geology	
ня		None Observed			0.5		СН	GRAVELLY CLAY: High plasticity, red brown 8 approx 35% fine to coarse angular basalt fragn to 150mm in size.  MW BASALT: Dark brown stained pale brown; to very high strength; numerous XW zones con hard, red brown, high plasticity clay; breaks into fragments from 20mm to 200mm in size on excavation.	high	D-M	Н			MW BASALT RS/XW BASALT	
					1. <u>5</u> - 2. <u>0</u> -			Near refusal at 1.4m Test pit CG10 terminated at 1.4m							

Sketch

ı									
- 1	method support				samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D disturbed sample		syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Excavation No.

Date completed:

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

**CG11** 

22.5.2008

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment ty	pe and	d model: (	CASE - 450	nm Bud	ket	Pit Orientation: Easting:	345940	m	R.L	Surface:		
excavation di	imensi	ons: 3	3.5m long	0.7m w	/ide	Northing:	704194	4 m	dat	um: AHD		
excavatio	n info	rmation		mat	aterial substance							
120	support water	notes samples, tests, etc	deptl RL metre		classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	- 1	condition consistency/ density index	100 pocket 200 d penetro- 300 meter	structure and additional observations	unit geology	
HB H	N None Observed		1. <u>5</u> 2. <u>0</u>		D-M	MIXTURE OF SOIL (30%) AND ROCK (70%): Soil gravelly clay; medium to high plasticity, dark brown fine to coarse angular basalt gravel; rock comprise basalt fragments to 400mm in size (angular).  HW BASALT: Dark brown and red brown (stained medium to high strength; numerous XW zones comprising hard, red brown, high plasticity clay; bre into fragments up to 100mm in size (but mainly up 50mm) on excavation.  Near refusal at 1.0m Test pit CG11 terminated at 1m	n; es	M		Corestones in a soil matrix	MW BASALT RS/HW BASALT	

Sketch

	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket		D disturbed sample		syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
8			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
0			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ξ			water inflow					D	dense
For			water outflow	l				VD	very dense



**CG12** 

Excavation No.

Sheet 1 of 1

**GEOTKPAR01416AA** Project No: 22.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED Date started: Principal: 22.5.2008 Date completed:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equipment	type	and	model: 0	CASE	- 450m	m Buc	ket	Pit Orientation:	Easting:	34556	88 m		R	.L. Surface:		
excavation	dim	ensic	ons: 3	3.5m l	long (	).7m w	ide		Northing:	70420	)10 m		da	atum:	AHD	
excavat	ion	info	rmation			mat	aterial substance									
method 1 C penetration		water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle c colour, secondary and minor	components.		condition	consistency/ density index	100 pocket 300 by penetro-		structure and onal observations	unit geology
HB	X	None Observed			1.0 - - 1.5 - - 2.0		CL	GRAVELLY CLAY: Medium plastici and grey; 40% fine to coarse gravel basalt fragments to 150mm in size.  HW BASALT: Dark brown and red I medium to high strength; numerous comprising hard, red brown, high plainto fragments up to 100mm in size 50mm) on excavation.  Near refusal at 0.9m Test pit CG12 terminated at 0.9m	brown (stained) s XW zones asticity clay; bre	ilar 	D-M	Н				MW BASALT RS/XW BASALT

	method		support	notes, s	amples, tests	clas	sification symbols and	consisten	cy/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Şe	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
88			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



Excavation No.

Sheet 1 of 1
Project No: **GEOTKPAR01416AA** 

**CG13** 

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Principal: Date completed: **22.5.2008** 

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equip	ment t	type	and	model: (	CASE	- 450m	ım Buc	ket	Pit Orientation: Easting:	340	360 m		R.I	Surface:	
excav	ation/	dime	ensic	ns: 3	3.5m l	ong (	0.7m w	ide	Northing:	704	10874 m	١	da	tum: AHD	
exc	avati	on i	nfo	mation			mat	erial s	ubstance						
	benetration 5	support	water	notes samples, tests, etc	RL I	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics colour, secondary and minor components.		moisture condition	consistency/ density index	100 pocket 200 pocket 300 pocket 400 meter		unit geology
Н		N	None Observed			0. <u>5</u>		CL	GRAVELLY CLAY: Medium plasticity, dark brown and grey; 40% fine to coarse gravel; trace of ang basalt fragments to 150mm in size.  HW BASALT: Dark brown and red brown (staine medium to high strength; numerous XW zones comprising hard, red brown, high plasticity clay; in fragments to 80mm, but some to 300mm in size.	d);	D-M	VSt/ H			HW BASALT RS/XW BASALT
						1. <u>5</u> - - 2. <u>0</u> - - - 2.5			Near refusal at 1.4m Test pit CG13 terminated at 1.4m						-

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ε			water inflow					D	dense
Po			water outflow					VD	very dense



**CG14** 

Excavation No.

Date completed:

Sheet 1 of 1

**GEOTKPAR01416AA** Project No:

22.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED 22.5.2008 Date started:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equ	quipment type and model: CASE -					- 450m	m Buc	ket	Pit Orientation: Easting:	346525	m	R.L	Surface:	
exc	avation	dime	ensic	ons: 3	3.5m l	long (	).7m w	ide	Northing:	704082	3 m	dat	um: AHD	
ех	cavati	on i	nfo	rmation			mat	erial s	ubstance					
method	t penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture		100 pocket 200 d penetro- 300 meter	structure and additional observations	unit geology
H		N	None Observed			0. <u>5</u>		CL	<b>GRAVELLY CLAY:</b> Medium plasticity, dark brown and grey; 40% fine to coarse gravel; trace of angul basalt fragments to 150mm in size; some pockets a zones of (CH) CL, high plasticity, dark brown; up to 0.5mm in size.	and )	ИН		- - - -	RS/XW BASALT
			Nor			- - -			HW BASALT: Dark brown and red brown (stained) medium to high strength; numerous XW zones comprisng hard, red brown, high plasticity clay; bre into fragments up to 100mm in size (but mainly up 50mm) on excavation; near refusal at 0.90m,	eaks			-	HW BASALT
						1.0 - - 1. <u>5</u> - 2.0			Near Refusal at 0.9m Test pit CG14 terminated at 0.9m				-	
						2.5							-	

Sketch

	method		support	notes, s	amples, tests	clas	sification symbols and	consistenc	y/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>ss</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO			1.			$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



**COOPERS GAP WINDFARM** 

Excavation No.

Logged by:

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

IS

**CG15** 

ient: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Principal: Date completed: **22.5.2008** 

Test pit location: See Figure 1 Checked by:

equ	uipment type and model: CASE -					- 450m	m Buc	ket	Pit Orientation: Easting:	345	657 m		R.L	. Surface:	
	avation				3.5m l	ong (			Northing:	703	9508 m	1	dat	um: AHD	
ex		on i	nfo	rmation			mat	erial s	ubstance					ı	
method	5 penetration	support	water	notes samples, tests, etc	RL I	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics colour, secondary and minor components.		moisture condition	consistency/ density index	100 200 x pocket 300 x penetro- 400 meter	structure and additional observations	unit geology
ВН			None Observed			0.5		CL	GRAVELLY CLAY: Medium plasticity, dark brown and grey; 40% fine to coarse gravel; trace of ang basalt fragments to 150mm in size; some pocket: (CH) clay, red brown and orange brown; to 1mm size.  HW BASALT: Dark brown and red brown (stainer medium to high strength; numerous XW zones comprisng hard, red brown, high plasticity clay; m fragments to 150mm, but some to 300mm in size	ullar s of in  d);	D-M	н		- - - - - - - - - - -	MW BASALT RS/XW BASALT
						1. <u>5</u> - 2. <u>0</u> - 2.5			Near refusal at 1.40m Test pit CG15 terminated at 1.4m					- - - - - - -	-

Sketch

Project:

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Sheet 1 of 1

Excavation No.

Date completed:

Project No: **GEOTKPAR01416AA** 

22.5.2008

**CG16** 

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equi	quipment type and model: CASE - 450m cavation dimensions: 3.5m long (							ket	Pit Orientation: Easting:	345	459 m		R.L	Surface:	
					3.5m l	ong (	).7m w		Northing:	703	9554 m	1	dat	um: AHD	
ex	cavati	on i	nfo	rmation			mate	erial s	ubstance						, Ц
method	5 penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.		moisture condition	consistency/ density index	100 200 x pocket 300 x penetro- 400 meter	structure and additional observations	unit geology
ВН		N	None Observed			0. <u>5</u>		CL/CH	GRAVELLY CLAY: Medium plasticity, dark brown and grey; 40% fine to coarse gravel; trace of angubasalt fragments to 150mm in size; some pockets dark brown CH  HW BASALT: Dark brown and red brown (stained medium to high strength; numerous XW zones comprising hard, red brown, high plasticity clay; m fragments to 100mm in size some to 300mm; plat fragments with orentation down slope.  Refusal at 1.3m	ular s of	D-M	Fb/VH		-	HW BASALT XW BASALT RS
						1. <u>5</u>			Test pit CG16 terminated at 1.3m					- - - - - - -	-

Sketch

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Sheet 1 of 1

Excavation No.

Project No: **GEOTKPAR01416AA** 

**CG17** 

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Principal: Date completed: **22.5.2008** 

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equ	quipment type and model: CASE - 4					- 450m	m Buc	ket	Pit Orientation: Easting:	: 3	345354 m		R.L	Surface:	
exc	avation	dim	ensic	ons: 3	3.5m	long (	).7m w	ide	Northing	g: 7	7039572 m		dat	um: AHD	
ex		on i	nfo	rmation			mat	erial sı	ubstance						
method	5 penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristi colour, secondary and minor component	ts.	moisture	consistency/ density index	100 pocket 200 pocket 300 popenetro- 400 meter	structure and additional observations	unit geology
ВН		N	None Observed			0. <u>5</u>	0000		MIXTURE OF SOIL (25%) and ROCK (75%): 8 gravelly clay; medium plasticity, dark brown; rocomprises rock fragments to 600mm in size; at basalt.	ock					HW-MW BASALT
						1.0			Near refusal at 0.7m Test pit CG17 terminated at 0.7m						

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ε			water inflow					D	dense
Po			water outflow					VD	very dense



**CG18** 

Sheet 1 of 1

Excavation No.

Date completed:

**GEOTKPAR01416AA** Project No:

22.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED 22.5.2008 Date started:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equipment	type	and	model: 0	CASE	- 450m	ım Buc	ket	Pit Orientation: Easting:	34	15216 m		R.L	. Surface:	
excavation	dim	ensic	ons: 3	3.5m l	ong (	).7m w	ide	Northing:	70	)39561 m	ı	dat	um: AHD	
excavati	ion	info	rmation			mat	erial s	ubstance						
method 1 7 8 9 9 9 1 8 1 9 1 9 1 1 1 1 1 1 1 1 1 1	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics colour, secondary and minor components.	5,	moisture condition	consistency/ density index	100 200 A pocket 300 B penetro- 400 meter	structure and additional observations	unit geology
Н	Z				0. <u>5</u>		СН	CLAY: High plasticity, dark brown; some fine to coarse basalt gravel; some zones medium plastic clay; trace of boulders to 300mm in size  CLAY: High plasticity, red brown.	city	D-M	Н		- - - -	RESIDUAL SOIL
		None Observed			1. <u>0</u>		CL	SILTY CLAY: Low to medium plasticity, orange brown and pale brown; trace of fine to coarse ba gravel.	. — — isalt	D	Fb/H		- - - - -	XW BASALT
					1. <u>5</u>	8 9	GC	CLAYEY GRAVEL: Fine to coarse, angular, oran brown and pale brown; ~40% low plasticity fines; some angular basalt fragments to 150mm in size	;	D	VD		- - - - -	XW-HW BASALT
					2.5			Test pit CG18 terminated at 2.1m					-	-

Sketch

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ε			water inflow					D	dense
Po			water outflow					VD	very dense



Excavation No.

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

**CG19** 

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Principal: Date completed: 22.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment	type	and	model: (	CASE	- 450m	m Buc	ket	Pit Orientation:	Easting:	3450	)222 m		R.I	Surface:		
excavation	dime	ensio	ns: 3	3.5m l	long (	).7m w	ide		Northing:	7039	564 m		da	tum:	AHD	
excavati	ion i	nfo	mation			mat	erial sı	ıbstance								
method 1 5 penetration		water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle of colour, secondary and minor	components.		moisture condition	consistency/ density index	100 pocket 200 d penetro- 300 w meter		ructure and nal observations	unit geology
BH	N	served			-		CL	GRAVELLY CLAY:Medium plastici and grey; 40% fine to coarse grave basalt fragments to 30mm in size.  HW BASALT: Dark brown and red	l; trace of angu	lar	D-M	Н				XW BASALT
		None Observed			0.5			medium to high strength; numerous comprisng hard, red brown, high plinto fragments up to 400mm in size 50mm) on excavation.	s XW zones asticity clay; bre	eaks					-	NW BASALT
					1.0			Refusal on basalt at 1.00m Test pit CG19 terminated at 0.95m							-	

	method		support	notes, s	amples, tests	clas	sification symbols and	consistenc	y/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>ss</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



Excavation No.

Date completed:

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

21.5.2008

CG2

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment	quipment type and model: CASE -						ket	Pit Orientation:	Easting:	345880 m	ı	R.L	. Surface:	
excavation	dim	ensic	ons: 3	3.5m ld	ong (	).7m w	ide		Northing:	7044046	m	datı	um: AHD	
excavat	ion i	nfo	rmation			mat	erial sı	ıbstance						
method 1 7 8 9 9 9 1 8 1 9 1 9 1 1 1 1 1 1 1 1 1 1	support	water	notes samples, tests, etc	RL r	depth metres	graphic log	classification symbol	material soil type: plasticity or particle cl colour, secondary and minor c	components.	moisture	consistency/ density index	100 pocket 200 pocket 300 popenetro- 400 meter	structure and additional observations	unit geology
HB HB	Z	None Observed			1. <u>5</u>		CH	CLAY: High plasticity, dark brown/ r fine to medium basalt gravel; Nume breaks into fragments 10mm-50mm hand pressure.  SILTY CLAY/ CLAY: Medium plastic plasticity, pale brown/ dark brown; v coarse basalt gravel.  CLAYEY GRAVEL: Fine to coarse, approximately 30% low plasticity fin fragments to 100mm in size.  HW-MW BASALT: Dark brown and medium to very high strength; with s dark brown low plasticity clay; break from 10mm - 150mm in size on excitations.	rous fissures; i in size under  city/ high with some fine to  pale grey; es; trace of base  red brown; some XW zone is into fragmen	o salt	1			HW-MW XW RS/XW BASALT RESIDUAL SOIL BASALT
					2.5								- - - -	_

Sketch

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e ne	E	excavator	refusal	Е	environmental sample	D	dry	H	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ε			water inflow					D	dense
Po			water outflow					VD	very dense



See Figure 1

Test pit location:

Sheet 1 of 1

Excavation No.

Checked by:

Project No: **GEOTKPAR01416AA** 

IS

**CG20** 

INVESTEC BANK (AUSTRALIA) LIMITED 22.5.2008 Date started:

Principal: 22.5.2008 Date completed:

**COOPERS GAP WINDFARM** Project: Logged by:

equipment type and model: CASE - 450mm Bucket Pit Orientation: Easting: 346086 m R.L. Surface: N I o utlo i o

excavation dime	excavation dimensions: 3.5m long					de	Northing:	7040163	m	datı	um: AHD	
excavation i	nform	nation			mate	erial su	ıbstance					
method  2 penetration support		notes samples, ests, etc	RL n	depth netres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	100 x pocket 200 v penetro- 300 w meter	structure and additional observations	unit geology
ВН	None Observed			- - 0. <u>5</u>		CL	GRAVELLY CLAY: Medium plasticity, dark brown and grey; 40% fine to coarse gravel; trace of angula basalt fragments to 150mm in size.	D-M	Н			XW BASALT
	None			- - 1. <u>0</u>		MW	MW BASALT: Dark grey and pale brown; high strength to very high strength; breaks into fragments of 40mm to 300mm on excavation; numerous clay seams and zone of variable thickness					MW BASALT
				- 1. <u>5</u>			Refusal at 1.1m Test pit CG20 terminated at 1.1m					-
				2. <u>0</u> -								- - - -
				2.5								-

ı	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syst	em	F	firm
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	moi	sture	VSt	very stiff
e ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
88			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO			l .			WL	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



Excavation No. **CG21** 

Sheet 1 of 2

Project No: **GEOTKPAR01416AA** 

INVESTEC BANK (AUSTRALIA) LIMITED Date started: 22.5.2008

Principal: Date completed: 22.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equ	uipment type and model: CASE - eavation dimensions: 3.5m lor					- 450m	m Buc	ket	Pit Orientation:	Easting:	340255	m	R.L	Surface:	
exc	avatio	n dim	ensi	ons: 3	3.5m l	ong (	).7m w	ide		Northing:	7044970	6 m	date	um: AHD	
ex	cava	tion	info	rmation			mat	erial sı	ubstance						
method	b penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle colour, secondary and minor	r components.	moisture		100 pocket 200 d penetro- 300 meter	structure and additional observations	unit geology
ВН		N				0.5		СН	<b>GRAVELLY CLAY:</b> High plasticity, red brown; approximately 40% fine subangular gravel; some cobbles a 300mm.	e to coarse	I D-I	ИН			COLLUVIUM
								СН	CLAY: High plasticity, red brown; s coarse gravel.	some fine to	D-I	М		-	RESIDUAL SOIL
						1.0		СН	CLAY: High plasticity, yellow brow banded in part; trace of fine to coa pockets of white silt.	n and pale grey, irse gravel; some	, M			-	
			None Observed			1. <u>5</u>								-	- - - -
			Z			2.0								_	XW BASALT

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Excavation No.

2 of 2 Sheet

**GEOTKPAR01416AA** Project No:

**CG21** 

INVESTEC BANK (AUSTRALIA) LIMITED 22.5.2008 Date started:

Principal: 22.5.2008 Date completed: **COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equipmer	nt type	and	model: (	CASE	- 450m	m Buc	ket	Pit Orientation: Easting:	340255	m		R.	L. Surface:		
excavatio	on dim	ensic	ons: 3	3.5m l	ong (	).7m w	ide	Northing:	704497	6 m		da	itum:	AHD	
excava	ation	nfo	rmation			mate	erial sı	ubstance							
method  L  penetration	3 dns	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture		consistency/ density index	100 pocket 200 do penetro- 300 pometer		cture and al observations	unit geology
ВН	N				3. <u>0</u>		GC	CLAY: High plasticity, yellow brown and pale grey, banded in part; trace of fine to coarse gravel; some pockets of white silt. (continued)  GRAVELLY CLAY: Medium plasticity with numerou fragments of basalt.		1	Н			- - - - -	
					3.5 - 4.0 - 4.5 - 5.0			Test pit CG21 terminated at 3.3m						- - - - - - - - - - - - - - -	

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Sheet 1 of 1

Date completed:

Excavation No.

Project No:

**GEOTKPAR01416AA** 

**CG22** 

22.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED 22.5.2008 Date started:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equipment	equipment type and model: CASE -						ket	Pit Orientation:	Easting:	341	870 m		R.I	Surface:	
excavation	dim	ensic	ons: 3	3.5m l	ong (	).7m w	ride		Northing:	704	6404 m	l	da	tum: AHD	
excavati	ion	info	rmation			mat	erial s	ubstance							
method 1 8 penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle colour, secondary and mino	r components.		moisture condition	consistency/ density index	100 200 A pocket 300 B penetro- 400 meter		unit geology
ВН	N				_		CL	SILTY CLAY: Medium plasticity, d	ark brown.		D-M	Fb/VSt			
		None Observed			0. <u>5</u>		СН	CLAY: High plasticity, red brown; coarse angular basalt gravel.  GRAVELLY CLAY: High plasticity fine to coarse angular gravel.			M	Н			XW BASALT
					1. <u>5</u> - - 2. <u>0</u>		GC	GRAVELLY CLAY: Fine to coarse grey; with some cobbles and bould basalt to 300mm in size; resistance depth.  HW BASALT: Low to medium stream and red brown; some clayey zone	ders of vesicular e increasing with ength, dark brow s.	า		VD			HW X BASALT   X
1000000					- - 2.5			Test pit CG22 terminated at 2.1m							

Sketch

	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
8			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
0			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ξ			water inflow					D	dense
For			water outflow	l				VD	very dense



Excavation No.

Date completed:

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

21.5.2008

CG3

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equ	equipment type and model: CASE - excavation dimensions: 3.5m lor					- 450m	m Buc	ket	Pit Orientation: Easting:	346	131 m		R.L	. Surface:	
exc	avation	dime	ensic	ons: 3	3.5m l	ong (	).7m w	ide	Northing:	704	4909 m	l	dati	um: AHD	
ех		on i	nfo	rmation			mat	erial s	ubstance						
method	5 penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.		moisture condition	consistency/ density index	100 pocket 200 pocket 300 popenetro- 400 meter	structure and additional observations	unit geology
Н		N	None Observed			0. <u>5</u>		СН	CLAY: High plasticity, dark brown; trace of fine to coarse basalt gravel; numerous fissures; breaks i 10mm to 50mm in size fragments under hand pressure.  HW BASALT: Pale grey brown and grey; very low strength; fragmented; some zones residual soil a some zones MW rock of medium to high strength  HW-MH BASALT: Greg and pale grey red; mediuto very high strength; beaks along defects to form fragments; 20mm to 100mm in size.  Block size increasing to 50mm - 200mm.	nto v nd i.	D-M	Н		_	HW/MW BASALT HW RESIDUAL BASALT SOIL
						1. <u>5</u> - 2. <u>0</u> - 2.5			Near refusal at 1.35m Test pit CG3 terminated at 1.3m					-	- - - - - - - -

Sketch

	method		support	notes, s	amples, tests	clas	sification symbols and	consisten	cy/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Şe	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	Е	excavator	refusal	Е	environmental sample	D	dry	Н	hard
88			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO			1.			$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



Sheet 1 of 2

Excavation No.

Project No: **GEOTKPAR01416AA** 

CG4

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Principal: Date completed: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE - 450mm excavation dimensions: 3.5m long 0.7					- 450m	m Buc	ket	Pit Orientation:	Easting:	346080 m		R.L	. Surface:	
excavation	on din	nensi	ons: 3	3.5m lo	ong (	).7m w	ide		Northing:	7044945 n	1	datı	um: AHD	
excava	ation	info	rmation			mat	erial s	ubstance						
method 1 7 penetration	- 1 ∺	water	notes samples, tests, etc		depth netres	graphic log	classification symbol	material soil type: plasticity or particle colour, secondary and minor	r components.	moisture condition	consistency/ density index	100 pocket 200 pocket 300 pocket 400 meter	structure and additional observations	unit geology
T 12	3 0 N	None Observed		KL II	0.5 - 1.0 - 1.5		СН	CLAY: High plasticity, red brown; the basalt gravel, moist.  CLAY: Medium to high plasticity, purpose the basalt; moist and boulders.  CLAY: Medium to high plasticity, purpose the basalt; hard; moist and boulders.  GRAVELLY CLAY: Medium plastic some pale grey; some fine to coar traces of cobbles and boulders to the basalt; hard; moist and basalt; hard; har	traces of fine  pale brown and loist; trace of	D-M	VSt	10	- - - - - - - - - - - - - - - - - - -	XW BASALT RESIDUAL SOIL
					2. <u>0</u> - 2.5								- - - - - - -	×

	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard
8			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
0			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ξ			water inflow					D	dense
For			water outflow	l				VD	very dense



Sheet 2 of 2

Excavation No.

Project No: **GEOTKPAR01416AA** 

CG4

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Principal: Date completed: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE	E - 450mm Bucl	cket Pit Orientation:	Easting:	346080 m		R.L.	. Surface:	
excavation dimensions: 3.5m	long 0.7m wi	vide	Northing:	7044945 n	n	datu	ım: AHD	
excavation information	mate	erial substance						
123	depth depth depth metres	Soil type: plasticity or particl colour, secondary and min	characteristics, r components.	moisture condition	consistency/ density index	100 pocket 200 pocket 300 pocket 400 meter	structure and additional observations	unit geology
H N				D-M	Н			
	3. <u>0</u> 3. <u>5</u> 4. <u>0</u> 4. <u>5</u> 5.0	Test pit CG4 terminated at 2.6m						

L									
- [	method		support	notes, s	amples, tests	clas	sification symbols and	consistenc	y/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Rev.	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e	E	excavator	refusal	E	environmental sample	D	dry	Н	hard
5.2 Issue			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
Ö			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
Form			water outflow					VD	very dense



Excavation No.

Sheet 1 of 1

Project No: **GEOTKPAR01416AA** 

CG5

21.5.2008

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Principal: Date completed: 21.5

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE - 4 excavation dimensions: 3.5m long						450m	m Buc	ket	Pit Orientation: Eas	ting:	346020 m		R.L	Surface:	
					3.5m loi	ng 0	).7m w			thing:	7044967 m		dat	um: AHD	
ex		tion	info	rmation		_	mate		bstance						_
method	Denetration	support	water	notes samples, tests, etc	RL m	depth netres	graphic log	classification symbol	material soil type: plasticity or particle charact	nents.	moisture condition	consistency/ density index	100 pocket 200 d penetro- 300 w meter	structure and additional observations	unit
BH		Z	None Observed			0.5			MIXTURE OF SOIL AND ROCK: Basalt fr from 60mm to 300mm in size of high stren high strength (70%); in a matrix of very stil clay (CH) of medium platicity.  MW BASALT: Dark grey, stained pale bro broken into blocks to 400mm on excavation	gth to ve f, gravell	rry Y			Weathered in-situ core stones (sub angular to subrounded)	MW BASALT XW/MW BASALT
						1. <u>0</u> - - - 1. <u>5</u>	<i>``</i>		Test pit CG5 terminated at 0.9m					-	MW
						- 2. <u>0</u> - -								- - - -	

	method		support	notes, s	amples, tests	clas	sification symbols and	consistenc	y/density index
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.7	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	Е	excavator	refusal	Е	environmental sample	D	dry	Н	hard
<u>ss</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
For			water outflow					VD	very dense



Sheet 1 of 1

Excavation No.

Project No: **GEOTKPAR01416AA** 

CG6

Client: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Principal: Date completed: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE - 4 excavation dimensions: 3.5m long				- 450m	m Buc	ket	Pit Orientation:	Easting:	346014	m	R.I	Surface:		
excavation of	dimensi	ons: 3	3.5m lo	ng (	).7m w	ide		Northing:	704497	3 m	dat	tum:	AHD	
excavation	on info	rmation			mate	erial sı	bstance							
method 1 7 penetration	support	notes samples, tests, etc	RL m	depth netres	graphic log	classification symbol	material soil type: plasticity or particle cha colour, secondary and minor co	mponents.	moisture		100 x pocket 200 x penetro- 300 w meter		ucture and al observations	unit geology
	None Observed			1.0 - 1.5 			MIXTURE OF SOIL AND ROCK: Bas from 60mm to 300mm in size of high shigh strength (70%); in a matrix of ver clay (CH) of medium platicity.  MW BASALT: Dark grey, stained pale high strength; broken into blocks to 40 excavation.  Test pit CG6 terminated at 0.9m	strength to very stiff, gravell	ery ly	M I		Weathered stones (sub subrounded	angular to	MW BASALT XW-MW BASALT BASALT

ı									
- 1	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	Χ	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D	disturbed sample	syste	em	F	firm
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
ne	E	excavator	■ refusal	E	environmental sample	D	dry	Н	hard
<u>88</u>			water	R	refusal	M	moist	Fb	friable
5.2			water level			W	wet	VL	very loose
			on date shown			Wp	plastic limit	L	loose
GEO						$W_L$	liquid limit	MD	medium dense
Ē			water inflow	l				D	dense
Po			water outflow					VD	very dense



Sheet 1 of 1

Excavation No.

Project No: **GEOTKPAR01416AA** 

CG7

lient: INVESTEC BANK (AUSTRALIA) LIMITED Date started: 21.5.2008

Principal: Date completed: 21.5.2008

Project: COOPERS GAP WINDFARM Logged by: IS

Test pit location: See Figure 1 Checked by:

equipment type and model: CASE - 4					- 450m	m Buc	ket	Pit Orientation:	Easting:	346847 m		R.L	Surface:	
excavation	dime	nsio	ns: 3	3.5m l	ong (	).7m w	ide		Northing:	7045798 r	n	dat	um: AHD	
excavati	on i	nfo	mation			mat	erial s	ubstance						
method  1  No penetration		water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle cl colour, secondary and minor o	components.	moisture condition	consistency/ density index	100 pocket 200 pocket 300 popenetro- 400 meter	structure and additional observations	unit geology
Ha	Z	None Observed			0.5 1.0 1.5 2.0		MW	CLAY: High plasticity, red brown; fin angular basalt gravel; moist; some z gravel; trace of cobbles and boulder mover that the gravel; trace of cobbles and boulder mover that the gravel is a search of 40mm to 300mm on excavation; seams and zone of variable thickness.  Test pit CG7 terminated at 1.1m	rown; high s into fragmen numerous clay	·	VSt/H		-	

L									
- [	method		support	notes, s	samples, tests	clas	sification symbols and	consisten	cy/density index
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft
7.	BH	backhoe bucket	penetration	D disturbed sample			em	F	firm
Rev	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff
e	E	excavator	refusal	E	environmental sample	D	dry	Н	hard
88			water	R	refusal	M	moist	Fb	friable
5.2 Issue			water level			W	wet	VL	very loose
ö			on date shown			Wp	plastic limit	L	loose
GEO			1.			$W_L$	liquid limit	MD	medium dense
Ē			water inflow					D	dense
Form			water outflow					VD	very dense



1 of 1

Date completed:

Excavation No.

Sheet

Project No: **GEOTKPAR01416AA** 21.5.2008

CG8

21.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED Date started:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

Potham Individual Part of CLAY: High plasticity, red brown; fine to coarse angular basalt gravel; some zones of clayey gravel; trace of cobbles and boulders    MW   MW BASALT: Dark grey and pale brown; high   MW   MW BASALT: Dark grey and pale brown; high   MS   MS   MS   MS   MS   MS   MS   M	equipment type and model: CASE - 450r						- 450m	m Buc	ket	Pit Orientation:	Easting:	347826 m		R.L	. Surface:	
N   N   N   N   N   N   N   N   N   N								).7m w	ride		Northing:	7045896 n	า	datı	ım: AHD	
Solitype: plasticity or particle characteristics,   Solitype: plasti	excavation information							mat	erial s	ubstance						
angular basalt gravel; some zones of clayey gravel; trace of cobbles and boulders    O.5				water	samples,	RL	depth metres	graphic log		soil type: plasticity or particle of colour, secondary and minor	components.			6000 6000 6000 6000 6000		unit geology
	BH			None Observed			1.0 		СН	angular basalt gravel; some zones trace of cobbles and boulders  MW BASALT: Dark grey and pale is strength to very high strength; brea of 40mm to 300mm on excavation; seams and zones of variable thicknown to seams and zones of variable thicknown traces.	of clayey gravel	M I;	Fb/VSt			_

Sketch

	method		support	notes, samples, tests			sification symbols and	consistency/density index		
	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft	
	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft	
7.7	BH	backhoe bucket	penetration	D	D disturbed sample		em	F	firm	
Re	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff	
3	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff	
ne	Е	excavator	refusal	E	environmental sample	D	dry	Н	hard	
<u>ss</u>			water	R	refusal	M	moist	Fb	friable	
5.2			water level			W	wet	VL	very loose	
			on date shown			Wp	plastic limit	L	loose	
GEO			1.			$W_L$	liquid limit	MD	medium dense	
Ē			water inflow					D	dense	
For			water outflow					VD	very dense	



Excavation No.

Date completed:

Sheet 1 of 1 Project No: **GEOTKPAR01416AA** 

CG9

21.5.2008

INVESTEC BANK (AUSTRALIA) LIMITED 21.5.2008 Date started:

**COOPERS GAP WINDFARM** IS Project: Logged by:

See Figure 1 Test pit location: Checked by:

equipment type and model: CASE - 450n						m Buc	ket	Pit Orientation:	Easting:	84744	1 m		R	L. Surface:		
excavation dimensions: 3.5m long									Northing:	70459	68 m	l	da	atum:	AHD	
excavation information material substance																
notthod notes samples, tests, etc depth RL metres symplog classification symbol water symbol symbol method notes symbol notes						graphic log	classification symbol	material soil type: plasticity or particle c colour, secondary and minor	components.		condition	consistency/ density index	200 x pocket 300 vd penetro-		ucture and al observations	unit geology
Н	N	None Observed			1.5 2.0 2.5		MW	CLAY: High plasticity, red brown; fir angular basalt gravel; moist; some a gravel; trace of cobbles and boulde with the plant of cobbles and boulde strength to very high strength; breat of 40mm to 300mm on excavation; seams and zones of variable thickn. Test pit CG9 terminated at 0.85m.	zones of clayey rs  orown; high ks into fragmen numerous clay	- —	D-M	Fb/VSt				

Sketch

ı										
- 1	method		support	notes, samples, tests			sification symbols and	consistency/density index		
- 1	N	natural exposure	S shoring N nil	U <sub>50</sub>	undisturbed sample 50mm diameter	soil	description	VS	very soft	
- 1	X	existing excavation		U <sub>63</sub>	undisturbed sample 63mm diameter	base	ed on unified classification	S	soft	
7.	BH	backhoe bucket	penetration	D disturbed sample		syste	em	F	firm	
Še	В	bulldozer blade	1 2 3 4	V	vane shear (kPa)			St	stiff	
က	R	ripper	no resistance ranging to	Bs	bulk sample	mois	sture	VSt	very stiff	
e ne	E	excavator	refusal	Е	environmental sample	D	dry	Н	hard	
<u>88</u>			water	R	refusal	M	moist	Fb	friable	
5.2			water level			W	wet	VL	very loose	
			on date shown			Wp	plastic limit	L	loose	
GEO			1.			$W_L$	liquid limit	MD	medium dense	
Ē			water inflow	l				D	dense	
Po			water outflow					VD	very dense	